

Article

Deformation Risk Assessment of the Lar Dam: Monitoring Its Stability Condition

Mehrnoosh Ghadimi ^{1,2,*} and Mohammadali Kiani ^{1,*}¹ Faculty of Geography, University of Tehran, Tehran 1417853933, Iran² Institute of Seismology, Department of Geosciences and Geography, University of Helsinki, FI-00014 Helsinki, Finland

* Correspondence: ghadimi@ut.ac.ir or mehrnoosh.ghadimi@helsinki.fi (M.G.); keyani@ut.ac.ir (M.K.)

Abstract: Dam stability is one of the most essential geotechnical engineering challenges. Studying the structural behavior of dams during their useful life is an essential component of their safety. Terrestrial surveying network approaches are typically expensive and time-consuming. Over the last decade, the interferometric synthetic aperture radar (InSAR) method has been widely used to monitor millimeter displacements in dam crests. This research investigates the structural monitoring of the Lar Dam in Iran, using InSAR and the terrestrial surveying network technique to identify the possible failure risk of the dam. Sentinel-1A images taken from 5 February 2015 to 30 September 2019 and TerraSAR-X (09.05.2018 to 16.08.2018) images were analyzed to investigate the dam's behavior. The InSAR results were compared with those of the terrestrial surveying network for the period of 1992 to 2019. The Sentinel-1 results implied that the dam on the left side moved over 8 mm/yr. However, the pillars to the left abutment indicated an uplift, which is consistent with the TerraSAR-X results. Also, the TerraSAR-X data indicated an 8 mm displacement over a three-month period. The terrestrial surveying showed that the largest uplift was 19.68 mm at the TB4 point on the left side and upstream of the body, while this amount was 10 mm in the interferometry analysis for the period of 2015–2020. The subsidence rate increased from the middle part toward the left abutment. The geological observations made during the ninth stage of the terrestrial surveying network indicate that there was horizontal and vertical movement over time, from 1992 to 2019. However, the results of the InSAR processing in the crown were similar to those of the terrestrial surveying network. Although different comparisons were used for the measurements, the difference in the displacement rates was reasonable, but all three methods showed the same trend in terms of uplift and displacement.

Keywords: dam failure; InSAR; TerraSAR-X; Sentinel-1A; dam stability; terrestrial surveying network



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1. Introduction

Embankment dams are the most typical form of dams in the world, accounting for 78% of all dams [1]. Embankment dams are responsible for 80% of dam failures worldwide, indicating their predominance. A dam failure happens when a section of the dam or its foundation moves or subsides, causing the dam to leak water. In many cases, dam failure results in the escape of large amounts of water, which can represent a serious threat to humans and/or assets sitting downstream [2]. A tailings pond failure can be attributed to a number of factors, including seepage, overtopping, earthquakes, and other geological events. The consequences of such an incident are significant, with numerous casualties and property losses, as well as environmental damage. The tailings—which contain a substantial quantity of heavy metal elements—can cause soil pollution and the destruction of vegetation [3]. Dams need to be adjusted to changing conditions in order to supply water and energy. Given that dams are a vital infrastructure, the effects of their collapse can be severe in terms of both economic and societal consequences [4,5]. The analysis of risk factors and the assessment of the risk of failure of tailings dams represent the initial steps in

the management of the risk of failure of such dams. Only by having a clear understanding of the risk of failure of tailings dams can we obtain accurate information on the risk and provide a basis for the prevention and control of disasters. Consequently, it is essential to conduct relevant research on the causes of failure of tailings dams [6].

Monitoring displacements in dams is vital to detect possible problems in the structures, helping in making an objective decision regarding possible restoration or, in the worst case, the destruction of the dam if it represents a serious risk to society [7].

Remote sensing is a technique for collecting information about the Earth's surface (including land and water) [8,9]. Also, remote sensing is an effective approach for investigating change detection, which is the process of identifying and measuring changes in an extensive variety of surface phenomena over time [10,11]. Interferometry synthetic aperture radar (InSAR) technologies, which have been tested in multiple studies, improve dam safety by providing timely settlement assessments at a high spatial resolution [12–16]. By processing long periods of SAR images, InSAR techniques can be improved to millimeter accuracy and used to efficiently monitor ground movements and small displacements of structures such as dams, buildings, and bridges [17].

In addition, InSAR has been shown to be a promising alternative to high-precision leveling. The small-baseline subsets (SBASs) method was the first to use distributed scatterers (DSs) to reduce the influence of decorrelation by limiting the geometric and temporal baselines [18].

One of the dam monitoring techniques is geodetic analysis, which measures the displacements of the dam crest depending on the reservoir level, with reference to measurements of angles and distances [19,20]. Even so, the traditional geodetic techniques are extremely difficult and time-consuming, requiring professional individuals to carry out measurements for days in each measurement period [17,21]. Furthermore, high-precision leveling has been employed to measure vertical displacements in various studies; however, this technique is limited by costly human resources and materials, as well as the high expenses of implementing long-term leveling campaigns on the dam's crest [22].

Therefore, establishing dam structural safety is required to quantify the risk associated with a dam reservoir system. In other words, for the system's initial state and various failure modes, it is important to evaluate the risk of loading events as well as the system reaction for a given load condition [23].

The Lar Dam lake was initially impounded in May 1980. After water level lowering, it appears that water escape is occurring. However, water escaping from the dam's embankment may cause internal material degradation. Water leakage at the Lar Dam is mainly caused by the existence of caves and tunnels in the underlying limestone strata [24,25].

The total flow rate from all springs indicated that leakage from the reservoir was about $16 \text{ m}^3/\text{s}$ [26]. As a result, the lake's water connects to the region's underground, which is more than a hundred meters below the riverbed [24].

The main purpose of the current study was to determine the displacement rate using a terrestrial surveying network and InSAR to prevent catastrophic events (such as the St. Francis dam catastrophe in the United States in March 1928) and to develop safety management plans for hydraulic infrastructures.

2. Case Study

The Lar Dam is located in the western part of the Central Alborz Mountain Chain (CAMC), southwest of the Damavand volcano. The dam was constructed across the Lar Valley [27]. The dam location can be seen in Figure 1. The average annual precipitation in the study area is ~600 mm, with snow accounting for over 60% of that total [28]. The Lar Dam is located 75 Km from Tehran and 100 Km from Amol. Detailed information on the Lar Dam is presented in Table 1 [29].

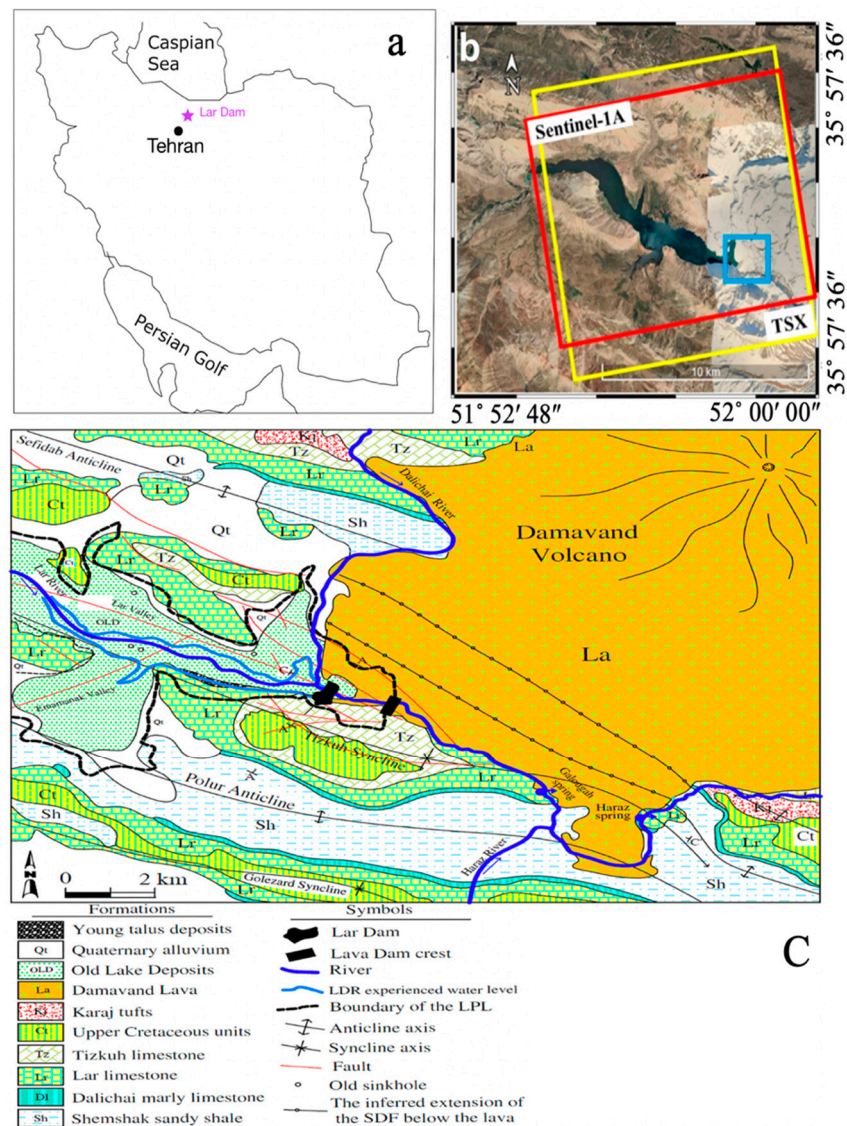


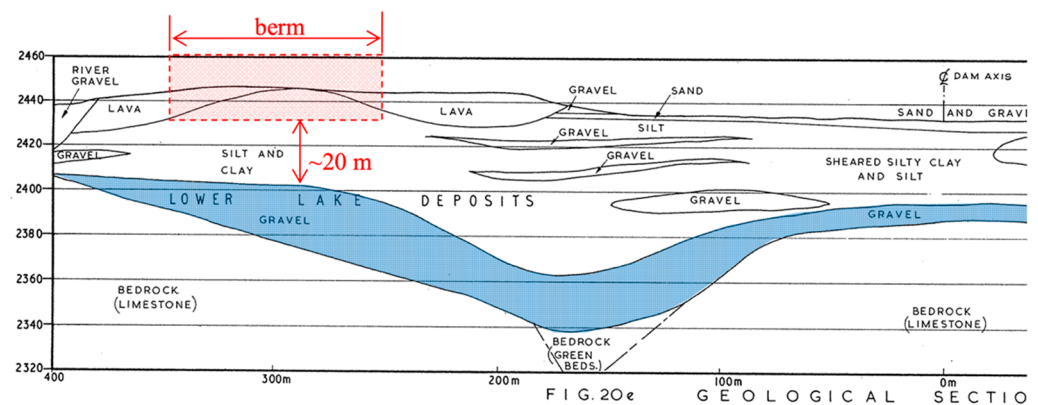
Figure 1. The Lar Dam location. (a) The red rectangle depicts the Sentinel-1A frame; (b) the yellow rectangle represents the TSX frame, while the blue rectangle represents the Lar Dam location; and (c) the Lar geological map [25].

Table 1. Characteristics of the Lar Dam.

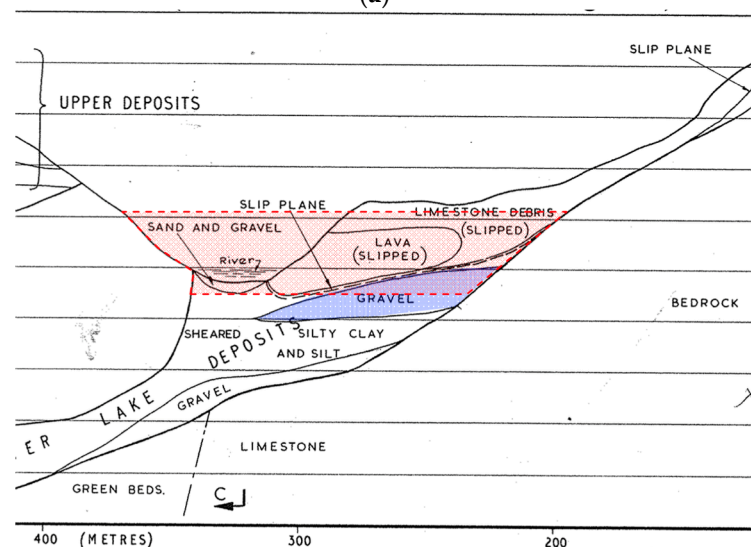
Type of Dam	Embankment
Year of completion	1980
Dam height above foundation (m)	107
Height from riverbed (m)	105
Dam crest width (m)	13
Dam crest length (m)	1170
Total water surface area of the reservoir (Km ²)	33.5
Reservoir width (Km)	1–6
Reservoir length (Km)	18
Total reservoir volume	960

The Lar Dam is a 1300 m long and 105 m high embankment dam built between 1974 and 1982. The dam contains an almost central core of silt and sandy silt, and filter and transition zones on the upstream and downstream core sides. The dam's shoulders of sandy gravel with slope inclinations range from 1:2.5 to 1:3.5. The cut-off is made up of a

32 m deep diaphragm wall in the alluvial foundation and a single-row grout curtain in the volcanic action and dolomite foundations. The usual functioning level is set at 2531 m above sea level [26]. The water level in the reservoir was growing slowly at the time, which was explained by the designer as the saturation of the void space in the soil formations several hundred meters thick beneath the reservoir. Also, the Lar Dam supplies 178 million m³/yr of water to the Jajrud basin, potable water to Tehran, and hydroelectric power [30]. The rock formations below the berm were further studied (Figure 2a,b). In the layout of the lava formation, it can be seen that the boundary of the lava flow is approximately at the crack location. The downward seepage occurs mainly through the lower lake deposits, assuming that the lava is comparable in impermeability. Hence, concentrated downward seepage occurs in the deposits and also at the boundary between the deposits and the lava, which becomes visible at the surface as an eroded line [29].



(a)



(b)

Figure 2. Geological section of the dam foundation: (a) lava flow at crack location; (b) erosion caused circular-shaped holes on the berm along the right bank [28].

The internal erosion of the materials caused by this gradient results in berm, sinkholes, fissures, and rock collapse (Figure 3).



Figure 3. A longitudinal crack formed on the berm at a height of 2462 m, with water leakage into the crack.

According to Figure 3, it can be concluded that downward flow and erosion toward more permeable zones (gravel pads or rock fractures) occur. It can be noted that the berm is partly founded on coarse-grained river alluvium. The downward seepage can cause the erosion of the overlying finer-grained soils into the coarse alluviums, which are free-draining to the underlying limestone [28].

At the beginning of the water extraction, the water leakage ranged between 3 and 13 (m^3/s). The dam's right and left abutments were built on a limestone formation and alluvial deposits (generated from a lava layer originating from the Damavand volcano), respectively. Reduction in the lake water level has been caused by karst channels in the limestone structure [30]. Two different water escape locations are depicted in Figure 4.



Figure 4. The water escape locations near the opening pumping tunnel in the Haraz Valley (a) and (b) the Galugah Valley.

High leakage was found in the range of 7.7 to 10.8 m^3/s , and the discharge volume from springs downstream of the Lar Dam (Haraz and Galugah) increased [24].

According to the SETEC Compony results [29], a section of the reservoir has the potential for leakage of water of over 1 million m^2 . The direction of groundwater flows and geological features were detected by hydrological investigations based on piezometric analysis (Figure 2). Water escaped from the Galugah and Haraz springs when the secondary reservoir was created beneath the reservoir's bed elevation (Figure 4).

Obvious evidence of deformation can be seen on the riprap layers of the Lar Dam in Figures 5 and 6.



Figure 5. The blue circles on the riprap layer of Lar Dam represent deformations.



Figure 6. Vertical movement of the riprap layer in the Lar Dam.

3. Materials and Methods

3.1. InSAR Processing

In this research, we studied two stacks of TerraSAR-X (TSX) images (10 ascending, with an incidence angle of 34.3° , and 8 descending, with an incidence angle of 32.8°). Spotlight data in the X-band spectrum were obtained from May to August 2018 (Table 2) and images and Sentinel-1A images from 5 February 2014 to 30 September 2019 (134 images) in the C-band, with an incidence angle of 32.3° (Table 3). We processed the TerraSAR-X and Sentinel-1 data using two different approaches (StaMPS and GMTSAR), respectively.

InSAR techniques are commonly employed to monitor small and local deformation data in engineering structures [31]. It is important to note that the small baseline method analysis used in StaMPS classifies single-look coherent pixels based on single-look images [32].

Table 2. Detailed information on the TerraSAR-X images.

Temporal Coverage	Orbit	Mode	Resolution (m)	Size	Incidence Angle	Heading Angle	Band
9.5.2018–16.8.2018	Descending	Spotlight	1	10 × 5	34.3	350	X
10.5.2018–17.8.2018	Ascending	Spotlight	1	10 × 5	32.8	190	X

Table 3. Detailed information on the Sentinel-1 images.

Temporal Coverage	Orbit	Mode	Resolution (m)	Swath (Km)	Incidence Angle	Heading Angle	Band
5 February 2015–30 September 2019	Ascending	IW	20 × 5	250	34.3	32.3	C

3.1.1. TerraSAR-X Processing

We analyzed the TSX spotlight data using the SBAS technique provided in the StaMPS 3.3b1 time-series software [7] and improved the spotlight data by [33,34].

We ordered the TSX images during the summer months (obtaining noise-free images). The TSX data used in this research included 18 images taken by the German TerraSAR-X mission in Spotlight mode. The TSX images were processed using the StaMPS software.

The differential interferometric phase results were obtained from the contributions of five phase components: topography, displacement, atmosphere, orbital error, and noise. The topographic phase can be mainly removed using a known digital elevation model (DEM), and the other phase components (orbit error, noise, and atmosphere) can be approximated and removed using appropriate image stack processing, leaving only the displacement phase component. The differential phase components can be represented by Equation (1) [35]:

$$\varnothing_{\Delta t(x,r)} = \varnothing_{disp(x,r)} + \varnothing_{top(x,r)} + \varnothing_{atm(x,r)} + \varnothing_{orb(x,r)} + \varnothing_{n(x,r)} \quad (1)$$

The standard SBAS approach was analyzed with time-series data using multi-looked interferograms [31].

For small baseline processing, 10 ascending and 8 descending interferograms were generated using threshold-snaphu 0.3 and threshold-geo-code 0.1 on cropped SAR data. The interferograms were unwrapped using a 3D phase-unwrapping method [32], and the displacement time series was obtained using a least-squares inversion [7]. The atmospheric factor of the phase was eliminated using a linear connection between the topography and interferogram phases.

3.1.2. Sentinel-1 Processing

We processed an analysis of the Lar Dam using 134 ascending Sentinel-1, C-band (5.6 cm wavelength) images and single-look complex (SLC) images produced from the interferometric wide (IW) swath mode with vertical polarization. The spatial resolution (pixel size) was approximately 5 m × 20 m in the range and azimuth directions. The data were processed with the 5.8 version of the “Generic Mapping Tools Synthetic Aperture Radar” GMTSAR software [36] with additional post-processing by GMT [37].

We subtracted the topographic phase using the (~90 m) Shuttle Radar Topography Model (SRTM) digital elevation model [37]. GMTSAR first extracts the orbital information in the pre-processing step of raw data and then estimates the Doppler centroid. Following image focusing, all images were aligned with a predetermined primary image. As a co-registration accuracy of better than 0.01 pixels was required in the long-track direction, GMTSAR employed the exact orbits (aux_porb) for the initial co-registration and the enhanced spectral diversity (ESD) method to remove the co-registration error at bursts and achieve the desired precision [38].

Following this step, 860 interferograms were created in GMTSAR and corrected for the topography and orbital errors to produce flattened results. The flattened interferograms were then passed through a low-pass filter before SNAPHU was used to unwrap the phase. Finally, the unwrapped interferograms were geocoded. We additionally estimated and eliminated the phase ramps from all images that could be present due to residual orbital errors and long-wavelength atmospheric signals, as well as minimizing atmospheric signals by assuming a linear relationship with the topography. We used the SBAS approach

in StaMPS to calculate the LOS displacement rates and cumulative displacement. SBAS makes use of differential interferograms collected at satellite sites separated by a short time interval.

3.2. Geodetic Data

The terrestrial surveying network over the Lar Dam was set up on and in proximity to the dam, and three measurements were made in 1992, 2013, and 2019. It should be noted that using an extensive terrestrial surveying network increases the measurement accuracy [30]. The geodetic points' locations (on the dam network) are shown in Figure 7.

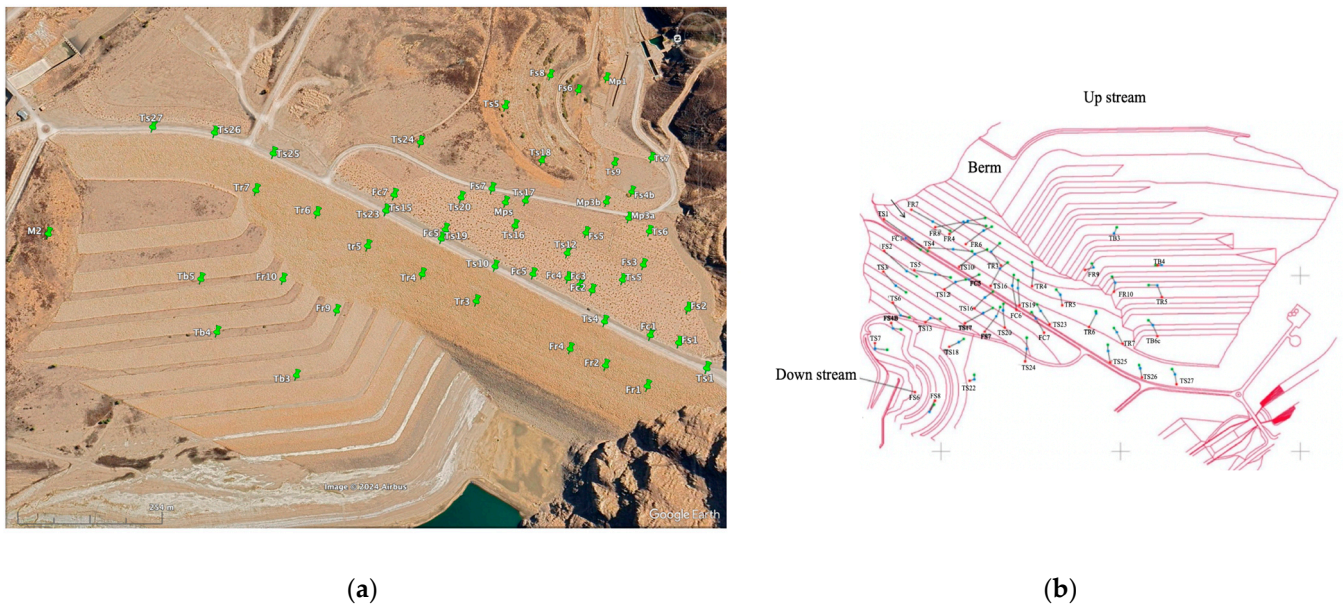


Figure 7. (a) Locations of the geodetic points in the Google Earth image. (b) Locations of the geodetic points and horizontal displacements collected in 1992, 2013, and 2019 [1].

4. Results

4.1. TerraSAR-X Results

The mean velocity data in the LOS direction were derived from TerraSAR-X. The ascending orbit findings indicate subsidence in the downstream part of the berm and on the dam's right side between 10.5.2018 and 4.7.2018. Figure 8a,b show the mean velocity in the LOS direction derived from the TerraSAR-X spotlight data in the ascending and descending orbits, respectively. Moreover, the output indicates that the rate of subsidence is ~ 6 mm/year. High-resolution images acquired by the TerraSAR-X with Sentinel-1 enabled us to obtain better and more reliable results and cross-validate the results. In ascending orbit, we could achieve more coherency and pixels.

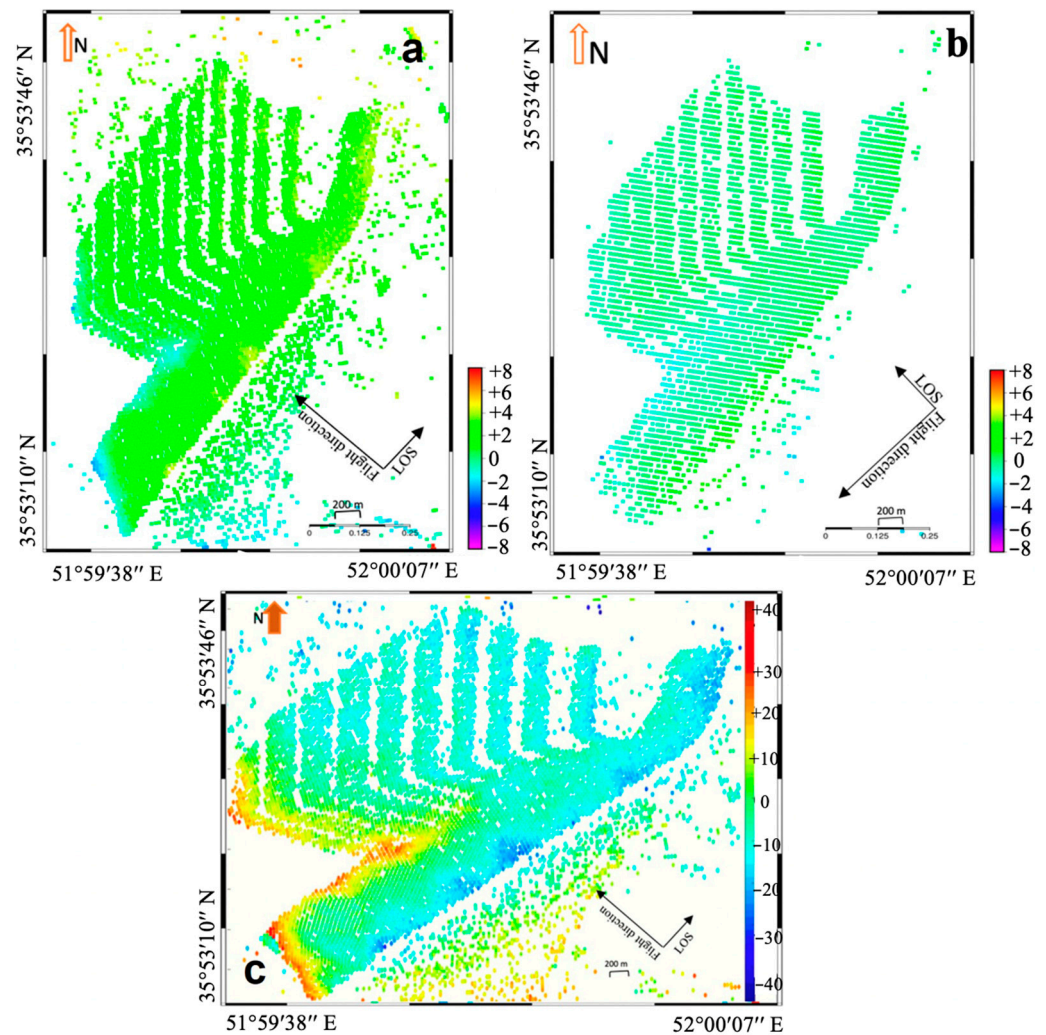


Figure 8. Mean LOS velocity obtained from TerraSAR-X during the period of May 2018 to August 2018, in the ascending (a) and descending orbits (b), respectively. (c) The mean linear velocity with TerraSAR-X during the period of May 2018 to August 2018 in the ascending orbit.

4.2. Sentinel-1A Results

The SBAS time series was created to analyze the Lar Dam's behavior. The LOS displacements were calculated using Sentinel-1A's ascending orbits. The Sentinel-1 analysis from 5 February 2015 to 30 September 2019 shows a maximum mean LOS velocity of 8 mm/y away from the satellite in the LOS direction, on the left side of the dam (Figure 9).

The coordinates of the observation pillars as well as the elliptical diameters of the error points at a 95% confidence level and flat displacement vector are depicted in Table 4.

This table shows that all of the pillars (1992–2019) had a horizontal displacement, including the M3 pillar, having the greatest displacement of 9.22 mm on the right abutment and upstream of the dam's body. Also, the results of the 2013–2019 period indicate the maximum displacement of 6.04 mm for the M1 pillar on the dam's left abutment. Several pillar locations in terrestrial surveying showed a vertical displacement between 1992 and 2019 (Table 5).

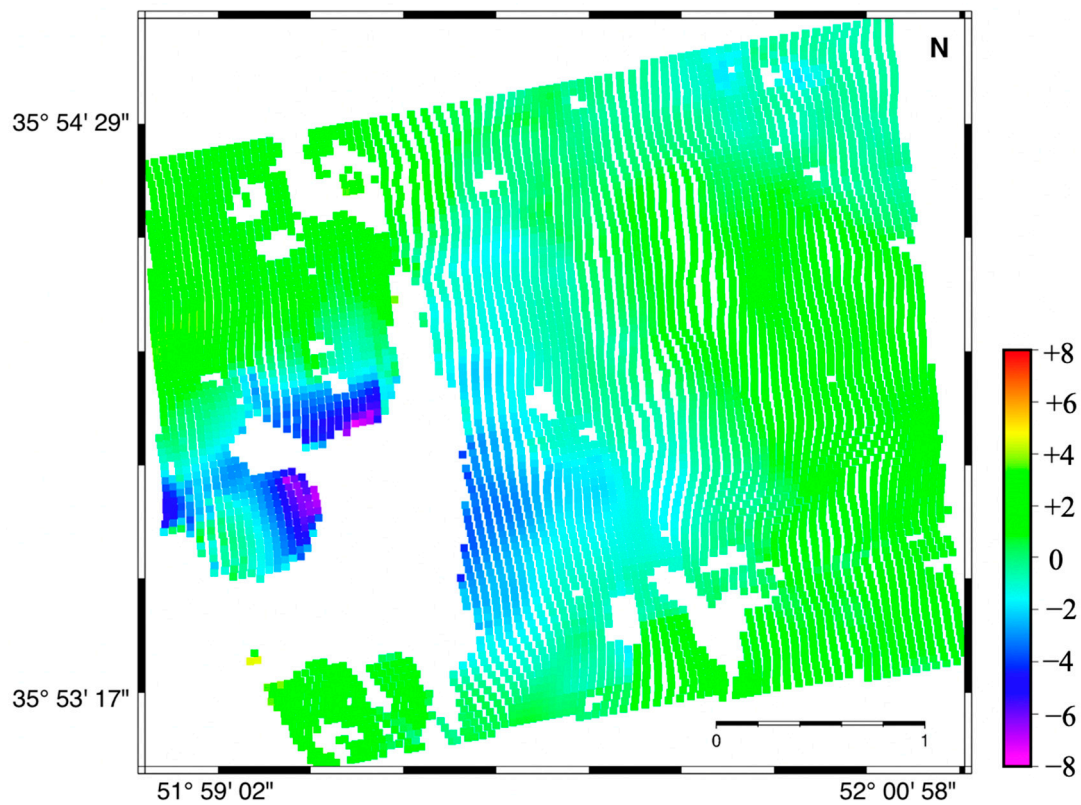


Figure 9. Mean LOS velocity for Sentinel-1 (2015–2019) processed using GMTSAR. Positive is toward the satellite.

Table 4. Horizontal displacements. Semi-minor and semi-major axes of the error ellipse are represented by a and b at a 95% confidence interval, respectively, and Az represents the azimuth of the major axis of the ellipse.

Point Name	Δx (mm)	Δy (mm)	D (mm)	Az (deg)	a (mm)	b (mm)	Phi (deg)
M5 (2013–2019)	−1.34	−1.68	2.15	218	1	0.9	16
M5 (1992–2019)	3.4	2.65	4.31	52	0.8	0.8	14
M6 (2013–2019)	3.02	−0.33	3.03	96	1.2	0.9	117
M6 (1992–2019)	−2.49	−4.54	5.18	208	1	0.9	120
M1 (2013–2019)	3.79	4.7	6.04	39	1	0.9	165
M1 (1992–2019)	−4.31	−6.19	7.54	215	0.9	0.8	31
M4 (2013–2019)	−1.88	−2.25	2.93	219	1	0.8	23
M4 (1992–2019)	4.55	5.27	6.96	40	0.8	0.8	10
M2 (2013–2019)	0.17	4.27	4.28	2	1	0.9	149
M2 (1992–2019)	−4.71	−5.69	7.39	219	0.9	0.8	20
M3 (2013–2019)	−3.76	−4.71	6.03	218	1.2	0.9	45
M3 (1992–2019)	3.57	8.5	9.22	22	1	0.8	42

Table 5. The vertical displacements along with the dimensions of the ellipse of the displacement error of the observational pillars at a confidence level of 99%.

Vertical Status	MΔz (mm)	Δz (mm)	MΔz (mm)	Δz (mm)	Point Name	Vertical Status	MΔz (mm)	Δz (mm)	Δz (mm)	MΔz (mm)	Point Name
	2013–2019		1992–2013				2013–2019		1992–2013		
Moved	3/9	−7.41	5.2	−43.26	FC1b	Fixed	0.0	0.00	0.0	0.00	L39
Moved	3/9	−6/73	5.2	−38.13	FS1b	Not Moved	0.3	−0.13	0.3	0.19	L38
Moved	3.7	14.39	5.0	−9.12	TR7b	Moved	3.5	10.79	4.7	−13.49	TS26b
Moved	3/8	13/17	5.1	−14.08	TR6b	Moved	3.6	11.59	4.9	−12.00	TS25b
Moved	3/9	10/36	5.1	−23.09	TR5b	Moved	4/1	−4/55	5.0	−24.04	FS9b
Moved	3.9	7.11	5.2	−47.82	TR4b	Moved/not moved	3.8	1.51	5.1	−39.08	FC7b
Moved	3.9	4.19	5.2	−54.71	TR3b	Moved	3.8	3.59	5.2	−53.82	TS19b
Moved	3.9	−0.79	5.2	−61.68	FR6b	Moved	3.9	3.12	5.2	−53.66	FC6b
Moved	3/9	−4/07	5.2	−68.03	FR4b	Moved	3.9	0.74	5.2	−62.00	TS15b
Moved	3/9	−5/27	5.2	−60.80	FR2b	Moved	3.9	−1.1	5.2	−193.01	FC5b
Moved	3/9	−3/21	5.2	−28.53	FR1b	Moved	3.9	−2.95	5.2	−66.96	TS10b
Moved	3/9	−7/34	5.2	−32.07	FS2b	Moved	3.9	−6.15	5.2	−200.61	FC4b
Moved/not moved	3/9	−3/35	5.2	−15.00	TS3b	Moved	3.9	−7.49	5.2	−457.81	FC3b
Moved	3/9	−4/79	5.3	−25.60	FS3b	Moved	3.9	−8.25	5.2	−153.45	FC2b
Moved	3/9	−4/65	5.2	−32.30	FS4b	Moved	3.9	−7.17	5.2	−72.87	TS4b
Moved	4/0	16/18	5.4	−19.43	FR10b	Moved/not moved	3/9	2/64	5.3	−27.53	MP5
Moved/not moved	4/0	−2/55	5.3	−18.59	MP3A	Moved/not moved	3/9	−1/57	5.2	−35.32	FS7b
Moved	4.1	−2.97	5.5	−9.48	MP1	Moved	4/0	1/1	5.3	−6.66	TS13b
Moved	3/9	−3/73	5.3	−18.56	TS1b	Moved	3/9	−2/47	5.3	−18.94	MP3b
Moved/not moved	4/0	−3.05	5.3	−15.64	TS6b	Moved/not moved	3/9	−3/47	5.3	−30.94	MP4
Moved	4/0	3/4	5.4	4.03	FS8c	Moved	3/9	−0/05	5.3	−47.46	TS16b
Moved	3/9	1/67	5.3	−35.78	FS7c	Moved	4/1	−5/55	5.5	−16.71	TS7b
Moved/not moved	3/9	−5/63	5.3	−30.70	FS2c	Moved	4/1	−4/55	5.5	−15.24	TS9b
Moved	4/1	−4/89	5.5	−13.11	FS4Bc	Moved	3/9	−4/65	5.5	−10.03	FS4Bb
Moved/not moved	3/9	−6/89	5.3	−50.29	FC1c	Moved	4/0	3/29	5.4	1.54	FS8b
Moved	3/9	−5/07	5.3	−50.51	FC4c	Moved	4/0	−3/05	5.5	−1.24	FS6b
Moved/not moved	3/9	4/88	5.2	−51.70	FC6c	Moved/not moved	3/9	0/33	5.3	−40.09	FC5c
Moved	4/1	15/42	5.5	−12.35	FR9b	Moved	3/7	10/15	4.8	5.23	M2
						Moved/not moved	4/3	0/96	5.8	14.5	M3
						Moved	4/0	4/27	5.4	8.80	M6

According to Table 4, some pillars had not moved. The vertical displacement on the left and right sides was mainly uplift and subsidence, respectively. The greatest uplift and subsidence occurred at point TB4 on the dam's left side (19.68 mm) and FC2b on the dam's right side (8.25 mm), respectively. Pillars M2 and M6 were located outside the dam, with 10.15 and 4.4 mm uplifts, respectively, and pillar M3 had not moved. The results of the terrestrial surveying network reveal that horizontal and vertical displacements occurred on the dam's body between 1992 and 2013 (Figure 10).

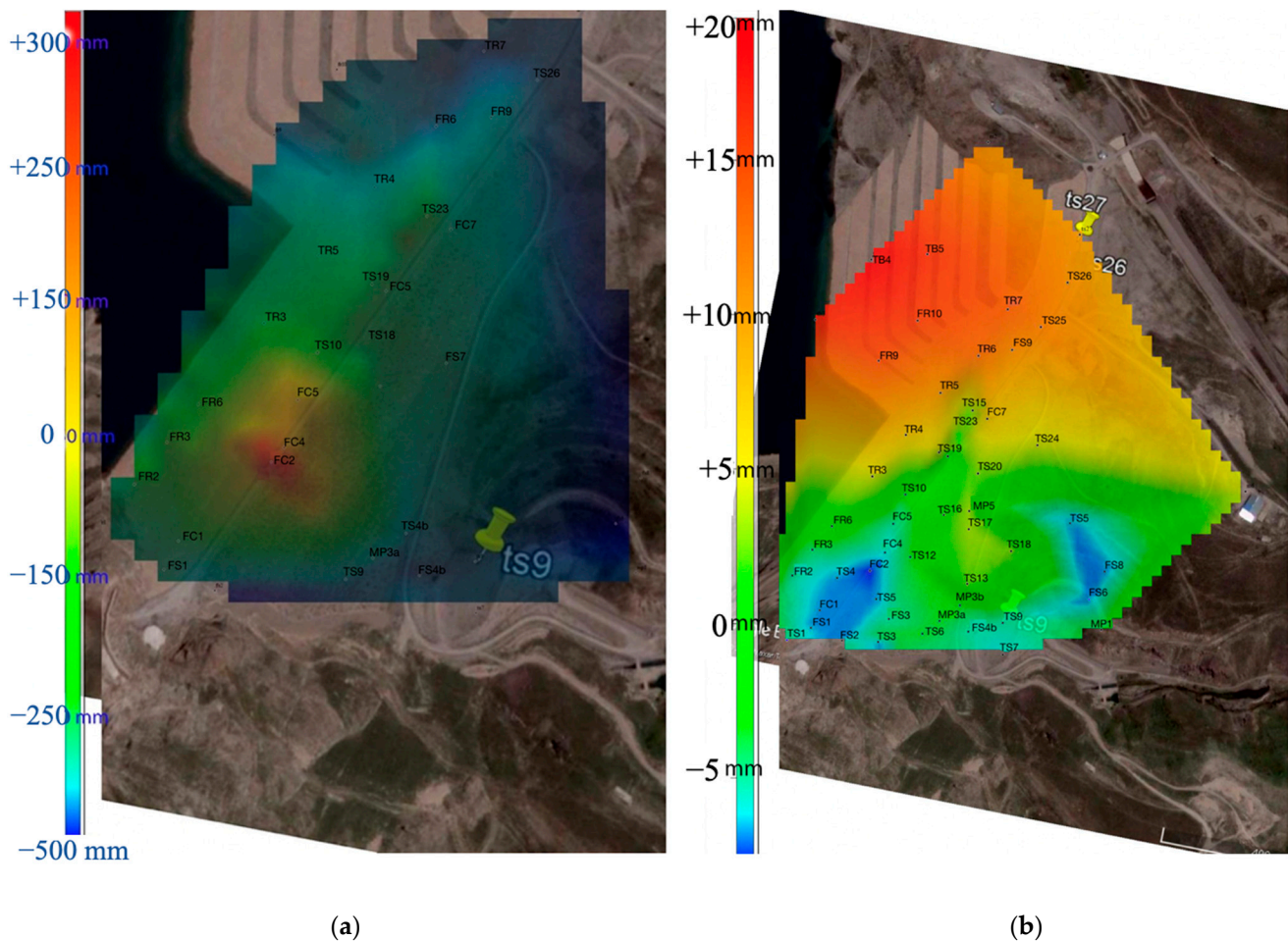


Figure 10. (a) The vertical deformation of pillar points on the Lar Dam in the period of 1992–2013. (b) The vertical deformation of pillar points on the Lar Dam in the period of 2013–2019.

It should be noted that the vertical displacement of the pillar points on the left abutment of the Lar Dam was mostly uplift, whereas on the right abutment, it was subsidence. The greatest elevation, 19.68 mm, relates to point TB4 on the left side and upstream of the dam body, and the highest subsidence, 25.8 mm, is linked to point FC2b on the right side and downstream of the dam crest. The geodetic observations in the ninth stage of micro-geodesy and the behavior measurement of the Lar Dam indicate that horizontal and vertical movements occurred on the dam body over the time period of 1992–2019.

In addition, the vertical displacement between the Sentinel-1 and TerraSAR-X images was consistent. As shown in Figure 11, the minimum and highest RMSE values between the TerraSAR-X and Sentinel-1 images were 0.87 in TS13 and 2.70 in FR9, respectively.

In the stage of 1992–2013 in the terrestrial surveying network, the process of vertical deformation was compatible with the natural behavior of the Lar Dam in the central part (the deepest section). In the clay core, the most settlement was observed, while it was reduced on the dam’s abutment [30]. In a recent terrestrial surveying network measurement, the left side of the dam was seen to be uplifting. In addition, Figure 10b depicts the pillars along the left abutment uplift between 2015 and 2020. As a result, both the terrestrial surveying network and InSAR technique verified an uplift on the left abutment of the dam’s body.

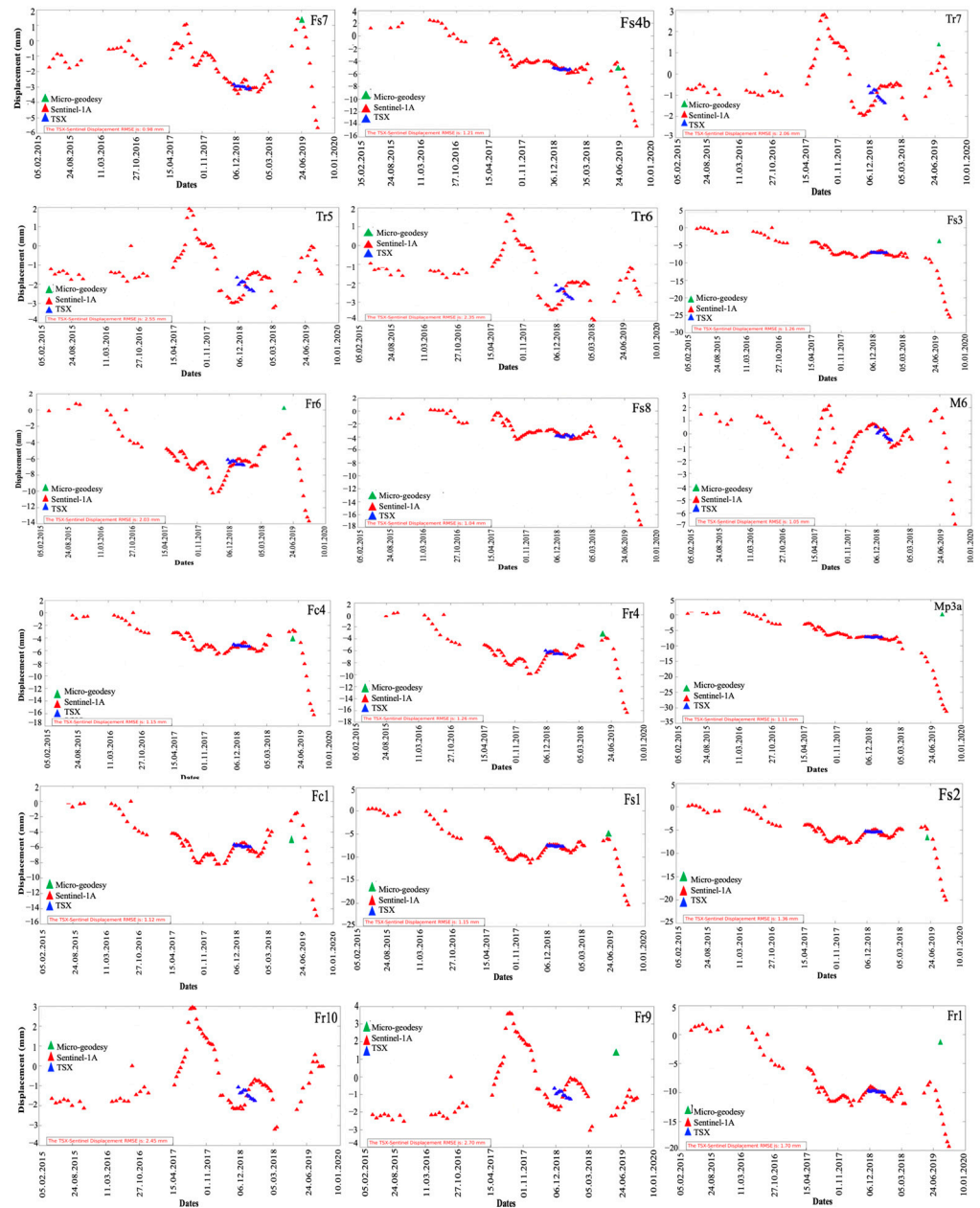


Figure 11. Cont.

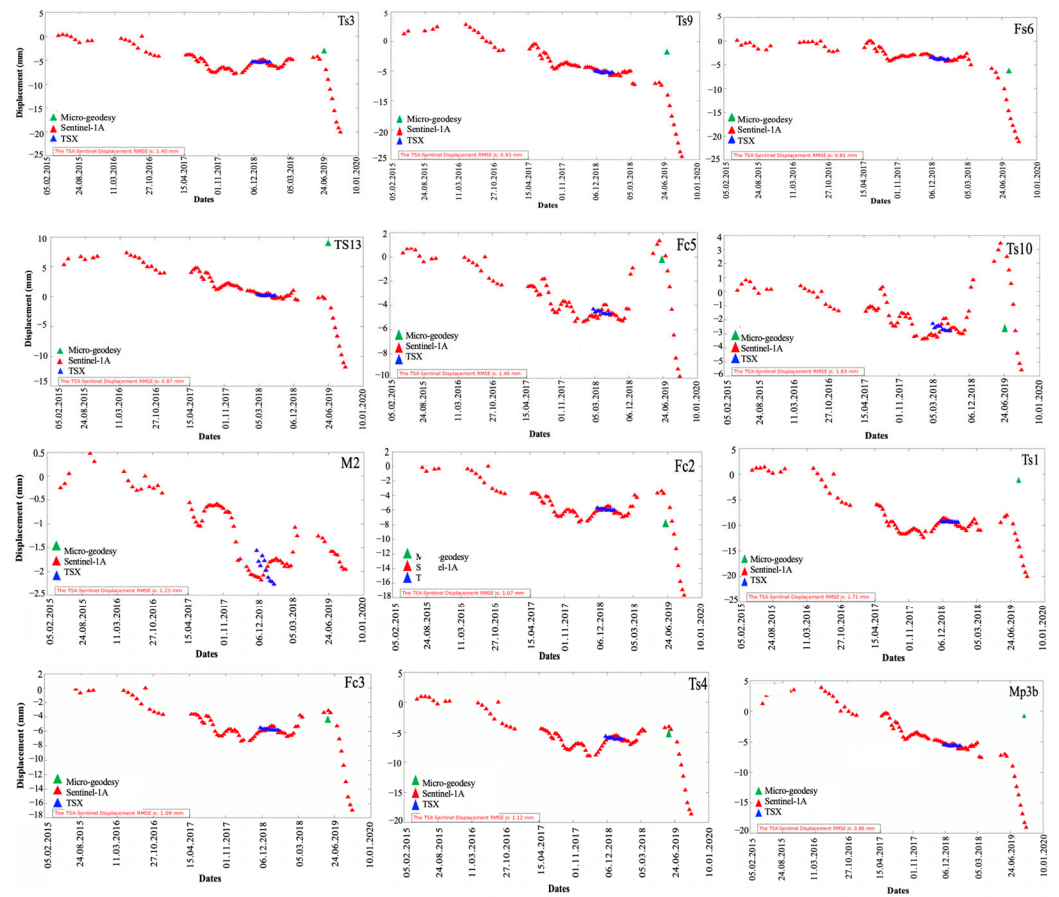


Figure 11. Comparison of vertical displacements in TerraSAR-X and Sentinel-1 images.

5. Discussion

For comprehensive research on dam displacement monitoring, various measurement techniques should be used [23].

In this research, we studied the risk assessment in the Lar Dam. The InSAR technique was used to study the structures with slow and long-term movements, such as dams. The high-resolution TerraSAR-X data revealed the dam deformation. In general, TerraSAR-X images have a spatial resolution of up to 1 m, which can improve the deformation monitoring capabilities. X-band data have already been shown to be appropriate for the high-resolution deformation monitoring of small areas [36]. The density of the pixels provides a better understanding and accurate description of deformation issues, providing more comprehensive deformation information [36,38,39]. For the processing, 10 ascending and 8 descending TerraSAR-X images from May to August 2018 were used. Obviously, having access to TSX images for a longer period of time could provide more results, but the acceptable results (Figure 11) and RMSE indicate the validity.

Due to the lack of data availability in the TerraSAR-X images, 134 Sentinel-1 images in ascending orbit were employed to compare with the terrestrial surveying network.

One of the most crucial techniques is the use of a terrestrial surveying network, which provides more precise information about the dam structure. To ensure precise monitoring with InSAR, it is necessary to validate the results with a terrestrial surveying network. In a recent terrestrial surveying measurement, the left side of the dam was seen to be uplifting. In addition, Figure 10 depicts the pillars along the left abutment uplift between 2015 and 2020. Also, the micro-geodesy points with the largest displacements and changes were compared with the results of the InSAR processing (Sentinel-1). As a result, both the terrestrial surveying network and the InSAR technique verified an uplift on the left abutment of the dam's body. As shown in Table 6, the displacement rate differed at various locations, but the terrestrial surveying points and InSAR results have a linear relation. Also,

the linear regression between the terrestrial surveying network and Sentinel-1 was 0.75 (Figure 12). It should be noted that the results were consistent. The RMSE between the terrestrial surveying network and Sentinel-1A was 2.19.

Table 6. Comparison of terrestrial surveying results with Sentinel-1A.

Pillar Point	Terrestrial Surveying Result (mm)	Sentinel-1A (mm)	Pillar Point	Terrestrial Surveying Result (mm)	Sentinel-1A (mm)
Ts13	+6	+3	Fs4b	−5	−14
M2	+10	+0.5	Tr6	+13.17	−3
Fc3	−6.33	−17	Fs2	−6	−20
Fc4	−5	−17	Fr1	−2.8	−20
Fc5	−1	−8	Tr7	14.39	+2
Fc2	−8	−18	Fs3	−4.8	−25
Ts4	−7	−20	Ts1	−3.64	−20
Fr4	−4	−17	Mp3b	−2.5	−20
Ts10	−3	−6	Mp3a	−2.5	−30
Fc1	−7.5	−14	Fr6	−0.79	−14
Fr10	+16	+2	Ts3	−3.35	−20
Fs7	+1.6	+3	Ts9	−4.5	−20
Tr5	+4.1	+1	Fs8	+3.29	−18
Fs1	−6.73	−20	M6	+4.27	−10
Fr9	+1.6	+1	Fs6	−7.4	−20

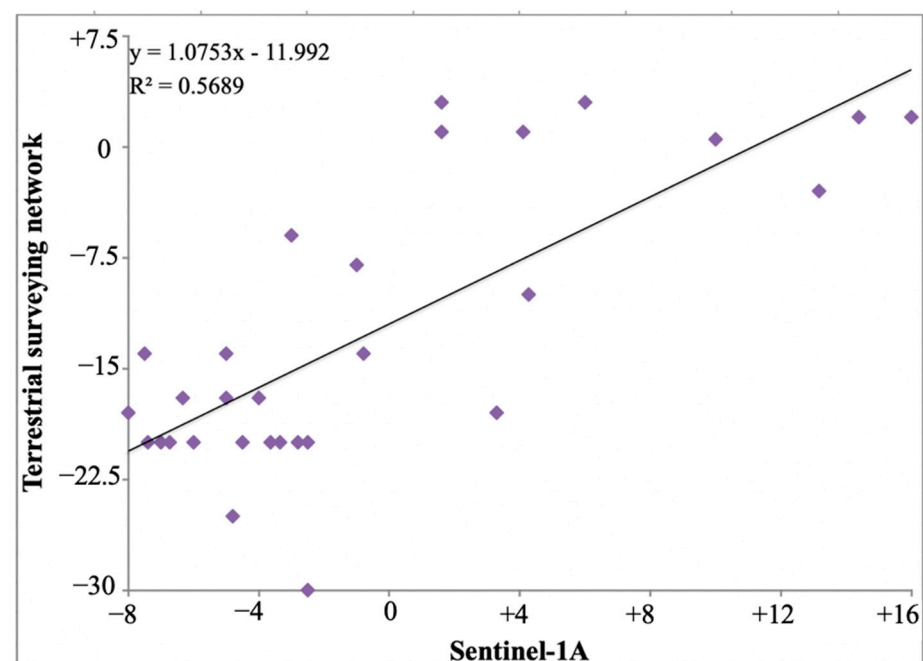


Figure 12. Linear regression between terrestrial surveying network and Sentinel-1.

According to the terrestrial surveying network, the maximum uplift of 19.68 mm was detected at point TB4 on the dam's left abutment; however, this was decreased to 10 mm by the InSAR method. The Sentinel-1 results show subsidence (Figure 9) on the dam's left side from 2015 to 2019, with inverse behavior in 2018 leading to an uplift. Based on the InSAR

results (Figure 9), the deformation range can be classified into three categories: the right abutment showed subsidence with 20 mm/y, and in the middle part of the dam to the left abutment (green color), it changed from subsidence to uplift during the last five years.

According to the reports of the Water Regional Organization [30], water escaping from limestone and karst passages in the reservoir and beneath the dam's body causes these deformations. As the water pressure increases, pore water from alluvial deposits infiltrates the limestone, and some of the limestone layers become filled with alluvial deposits, reducing the soil shear strength. Also, the hydraulic flow of the water forces the materials to function, and, over time, the alluvium beneath the dam's body and foundation erodes away, resulting in internal erosion [24]. The permeability of the foundation is vital for the stability of the dam structure. Poorly permeable materials raise the pore pressure and shear stress on the foundation [40,41].

As mentioned before by [41], the hydrology and geology of the dam's environment should be considered when choosing a dam foundation. As a result, there is the possibility of further subsidence, the unexpected collapse of the body, and the construction of a massive sinkhole.

In general, the precision achieved with InSAR was similar to that of the terrestrial surveying network, with a 0.75 correlation.

Additionally, this article includes a risk evaluation. Due to the karst region of the Lar Dam, it is obvious that there will be some risk of ground surface collapse, affecting all components.

In conclusion, the InSAR analysis (TerraSAR-X and Sentinel-1) and terrestrial surveying network revealed a significant displacement rate at the Lar Dam. The water leakage and field observations justify the variations. As a result, we can assume that the instability process continues to impact the Lar Dam. Further study should focus on the use of continuous InSAR processing and a ground surveying network to improve dam safety and reduce the associated hazards.

6. Conclusions

It is crucial to conduct a dam stability monitoring assessment. For instance, employing a terrestrial surveying network is a challenging, time-consuming, and costly process. As the data-monitoring instrumentation installed in the embankment dam has gradually lost its efficiency, and the information is not always reliable, it is necessary to consider alternative methods of monitoring. Satellite radar interferometry represents a valuable approach for the purpose of continuous monitoring, offering the advantage of scanning a large area in comparison with terrestrial surveying networks. In this research, it was demonstrated that the interpretation of InSAR results must be carried out with great scientifically based caution in order to prevent any misinterpretation of the superficial movement of the covering materials. The amount of displacement varies in specific places. Also, the terrestrial surveying network on the left abutment indicated an uplift, which was consistent with the TerraSAR-X results.

The Sentinel-1A images, captured between 5 February 2015 and 30 September 2019, and the TerraSAR-X images (9 May 2018 to 16 August 2018) were analyzed to investigate the dam's behavior. The InSAR results were compared with those of the terrestrial surveying network for the period between 1992 and 2019. The results of the Sentinel-1 images indicated that the dam on the left side had moved by more than 8 mm per year. Furthermore, the TerraSAR-X data indicated that the dam underwent 8 mm of displacement over a three-month period. The results of the terrestrial surveying indicated that the maximum uplift was 19.68 mm at the TB4 point on the left side and upstream of the body. In contrast, the interferometry analysis for the period of 2015–2020 revealed a maximum uplift of 10 mm. The high-resolution TerraSAR-X images offered precise displacement measurements, although the high cost of the TerraSAR-X images limited the period for which these images could be obtained. The geodetic observations in the ninth stage of

micro-geodesy and the behavior measurements of the Lar Dam indicated that horizontal and vertical movements occurred on the dam body over the time period of 1992–2019.

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